Makes the most of yourview

Balcony 1 system (0.74 kN) ***PAGE 1*** **(B1NB6060300150BP141117R)**

<u>Structural Calculations for Orbit (Balcony 1) system handrail (with & without internal reinforcing bar) using 60 x 60 x 5mm SHS posts & 300 x 150mm x 15 base plates</u>

Our ref: (B1NB6060300150BP141117R)

Date of original issue: Nov 2017
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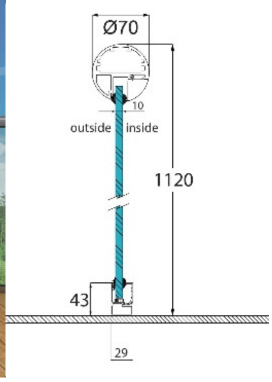
Balcony 1 Balustrade fixed between two walls

Balcony 1 Balustrade elevation with posts



Balcony 1 section Balcony 1

DESIGN TO EUROCODES & CURRENT BRITISH STANDARDS

Design standards:

EN 1990	Eurocode 0:	Basis of structural design.
EN 1991	Eurocode 1:	Actions on structures.
EN 1993	Eurocode 3:	Design of steel structures.
EN 1999	Eurocode 9:	Design of aluminium structures.
BS EN 1990:2002 + A1:2005	Eurocode:	UK National annex for Eurocode
BS 6180:2011	British Standard:	Barriers in and about buildings.

<u>Design loads:</u>

Occupancy class/es for which this design applies (Table 2: BS6180:2011)	=	Domestic and residential activities (i) & (ii) Office and work areas not included elsewhere (iii), (iv) & (v) Areas without obstacles for moving people and not susceptible to overcrowding (viii) & (ix)
Service load on handrail	= Q_k =	0.74 kN/m uniformly distributed line load acting 1100mm above finished floor level. (Table 2: BS6180:2011)
Service load applied to the glass infill	= Qk1 =	A uniformly distributed load of 1.0 kN/m^2
Point load on glass infill	= point load =	0.50 kN applied to any part of the glass fill panels

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Table 2 Minimum horizontal imposed loads for parapets, barriers and balustrades

Type of occupancy for part of the building or structure	Examples of specific use	Horizontal uniformly distributed line load (kN/m)	Uniformly distributed load applied to the infill (kN/m^2)	A point load applied to part of the infill (kN)
Domestic and residential activities	(i) All areas within or serving exclusively one single family dwelling including stairs, landings, etc. but excluding external balconies and edges of roofs	0.36	0.5	0.25
	(ii) Other residential, i.e. houses of multiple occupancy and balconies, including Juliette balconies and edges of roofs in single family dwellings	0.74	1.0	0.5
Offices and work areas not included elsewhere, including storage areas	(iii) Light access stairs and gangways not more than 600 mm wide	0.22	—	—
	(iv) Light pedestrian traffic routes in industrial and storage buildings except designated escape routes	0.36	0.5	0.25
	(v) Areas not susceptile to overcrowding in office and institutional buildings, also industrial and storage buildings except as given above	0.74	1.0	0.5
Areas where people might congregate	(vi) Areas having fixed seating within 530 mm of the barrier, balustrade or parapet	1.5	1.5	1.5
Areas with tables or fixed seatings	(vii) Restaurants and bars	1.5	1.5	1.5
Areas without obstacles for moving people and not susceptible to overcrowding	(viii) Stairs, landings, corridors, ramps	0.74	1.0	0.5
	(ix) External balconies including Juliette balconies and edges of roofs. Footways and pavements within building curtilage adjacent to basement/sunken areas	0.74	1.0	0.5

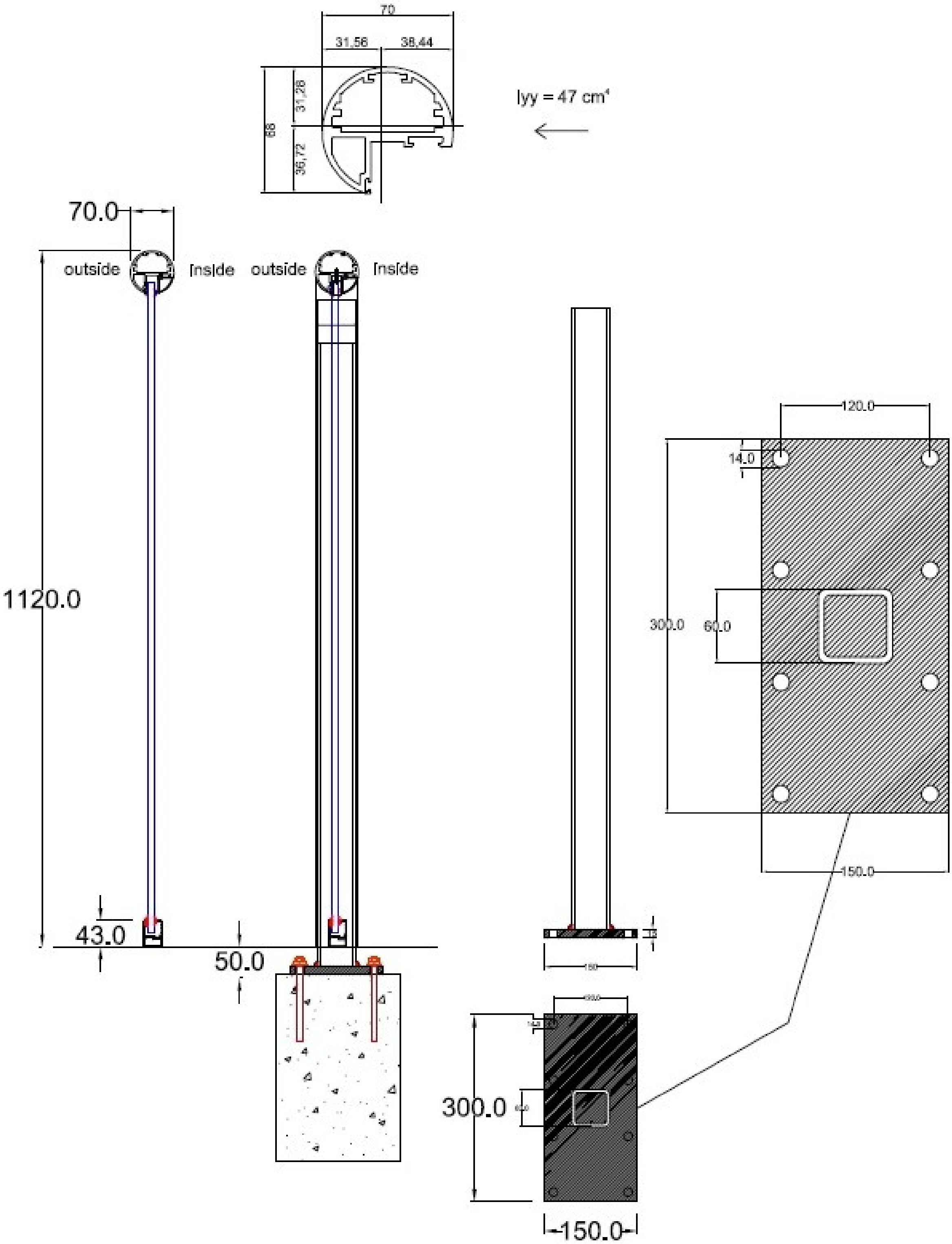
Table 2: BS6180:2011

- These loads are considered as three separate load cases. They are not combined.
- Factored loads are used for checking the limit state of static strength of a member.
- The service loads are multiplied by a partial factor for variable action $\gamma_{Q,1}$ of 1.5 to give the ultimate design load for leading variable action.

Deflection:

- All structural members deflect to some extent under load. Service loads are used to calculate deflections.
- The total displacement of any point of a barrier from its original unloaded position under the action of service loads is limited to 25mm.

Balcony 1 system (0.74 kN)



Section of Balcony 1 system, post detail and base plate detail

<u>Wind load parameters:</u>

Design wind loads are influenced by a number of variable factors. These include site location, site altitude above sea level, type of terrain, and height of balustrade above ground level.

These parameters and conditions are defined in BS EN 1991-1-4:2002 + A1:2010 '*Actions on structures – wind actions'* and UK National Annex to EN 1991-1-4:2002 + A1:2010. We have chosen to prepare a calculation based upon certain conditions, resulting in specific coefficients.

The formula applied results in an overall **characteristic wind pressure.** The design and calculations will be relevant not only to the conditions specified herein but to any combination of factors that results in a characteristic wind pressure that is equal to or less than the one specified in the calculations. Sites that have a characteristic wind pressure that exceeds **1.35 kN/m²** as determined in these calculations require separate consideration.

The selected wind load coefficients will cover the majority of sites in England and Wales, and are appropriate for 1100mm high balustrades of any length with or without return corners.

a) Sites located geographically within the 23m/sec isopleth in Figure NA 1 of the UK National Annex. This covers most of England and the eastern half of Wales.

b) Site altitude 100m maximum above sea level.

c) Top of balustrade located 35m maximum above ground level.

d) Site located in a coastal area exposed to the open sea, terrain category 0 of BS EN 1991 Table 4.1. This is the most severe exposure category. Smaller wind load coefficients apply to less exposed inland sites, terrain categories 1 to 1V.

e) Site located in country terrain or less than 1.0 km inside town terrain.

f) Sites with no significant orography in relation to wind effects. (ie. orography coefficient 1.0). Increased wind load coefficients apply to sites near the top of isolated hills, ridges, cliffs or escarpments.

g) Directional, seasonal, and probability factors are all taken as normal, for which the relevant coefficient is 1.0. This is a slightly conservative approach.

<u>Wind load design:</u>

Basic site wind speed	$V_{b\ map}$	=	23m/sec
Site altitude above sea level	A	=	100m
Handrail height above ground level	z	=	35m
Altitude factor	C_{alt}	=	$1.0 + (0.001 \times A)(10/z)^{0.2}$
		=	$1.0 + (0.1)(10/35)^{0.2}$
		=	$1.0 + (0.1)(0.7783)$
	say	=	1.08

<u>Wind load design</u> (cont):

Directional factor	C_{dir}	=	1.0
Seasonal factor	C_{season}	=	1.0
Probability factor	C_{prob}	=	1.0
Site wind speed	V_b	=	$V_{b\ map}$ (C_{dir} x C_{season} x C_{prob}) C_{alt})
		=	23m/sec x 1.08
		=	24.84m/sec
Site wind pressure	qb	=	0.613 $(Vb)^2$
		=	0.613 $(24.84)^2$
		=	378 N/m^2
Exposure factor	Ce (z)	=	3.50 (Figure NA 7)
Peak velocity pressure	qp	=	qb x Ce (z)
(characteristic wind pressure)		=	0.378 x 3.50
		=	1.323 kN/m^2
	say	=	**1.35 kN/m^2**
Wind load reaction on the handrail		=	1.35 kN/m^2 x 0.55
		=	0.74 kN/m
		=	same value as the specified imposed line load

For sites that come within the parameters listed on page 4 of these calculations, the specified imposed uniformly distributed line load on the handrail and the characteristic design wind loading on the handrail are approximately the same.

Wind pressure on the glass is greater than the specified overall design imposed UDL. Wind loading is therefore the controlling condition in terms of glass design.

Partial safety factor considering wind load as a separate leading variable action	γ_{Q1}	=	1.50
Ultimate design wind pressure		=	1.35 kN/m^2 x 1.50
		=	**2.025 kN/m^2**

<u>Summary of design loads:</u>

Element	**Service load**	**Ultimate load**
Horizontal imposed wind and line load applied to the handrail 1100mm above finished floor level (ie 1135mm above the top of the base).	0.74 kN/m	1.11 kN/m
Wind load on the glass	1.35 kN/m^2	2.025 kN/m^2
Point load applied to the glass in any position	0.50 kN	0.75 kN

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Section properties of handrail (with bar):

Material type	Extruded aluminium type 6063 T5		
Characteristic 0.2% proof stress	f_o	=	130 N/mm^2
Characteristic ultimate tensile strength	f_u	=	175 N/mm^2
Modulus of elasticity	E	=	70 000 N/mm^2
Shear modulus	G	=	27 000 N/mm^2
Moment of inertia about the y-y axis	I_{yy}	=	67 cm^4
Least elastic modulus about the y-y axis	W_{el}	=	17.43 cm^3
Partial factor for material properties	γ_{M1}	=	1.10
Shape factor (assessment)	α	=	W_{pl}/W_{el}
		=	1.2 say
Design ultimate resistance to bending about the y-y axis	M_{Rd}	=	$M_{o,Rd}$
		=	$\alpha W_{el} f_o / \gamma_{M1}$
		=	$\dfrac{1.2 \times 17.43\ cm^3 \times 130\ N/mm^2 \times (10)^{-3}}{1.1}$
		=	**2.472 kNm**

Section properties of handrail (without bar):

Properties as above except as follows:			
Inertia about the y-y axis	I_{yy}	=	47.0 cm^4
Least elastic modulus about the yy axis	W_{el}	=	12.227 cm^3
Design ultimate resistance to bending	M_{Rd}	=	$\alpha W_{el} f_o / \gamma_{M1}$
about the y-y axis		=	$\dfrac{1.2 \times 12.227\ cm^3 \times 130\ N/mm^2 \times (10)^{-3}}{1.1}$
		=	**1.734 kNm**

Handrail with bar: single span and corner system:

Design ultimate horizontal load on handrail	F	=	1.11 kN/m
Design horizontal moment on handrail between points of support, assuming simply supported spans (worst case) =	M	=	$\dfrac{F L^2}{8}$
Allowable span L between points of support based upon the moment capacity of the handrail		=	$\left[\dfrac{8 \times M_{Rd}}{F}\right]^{0.5}$
		=	$\left[\dfrac{8 \times 2.472\ kNm}{1.11}\right]^{0.5}$
		=	4.22m
	say	=	**4.0m**

In terms of bending capacity the handrail (with bar) can span up to **4.0m** simply supported between points of support. However for a single span simply supported handrail the service load deflection is limited to a maximum of **25mm**.

Handrail (with bar): Single span and corner system (cont):

Deflection (Δ) of a simply supported span (L) with an imposed UDL load (F)

$$\Delta = \frac{5 F L^4}{384 E I}$$

For a handrail span of 3.3m simply supported

$$\Delta = \frac{5\ (740 \times 3.3)\ (3300)^3}{384 \times 70\ 000 \times 67 \times (10)^4}$$

$$= 24.36\text{mm} \quad = \quad < \quad 25\text{mm} \quad \text{OK}$$

Therefore deflection limitations govern the allowable simply supported span of the handrail.

In order that calculated service load deflection shall not exceed 25mm the allowable simply supported span of the handrail is limited to **3.3m.**

Handrail (without bar): Single span and corner system:

Design ultimate horizontal load on handrail = 1.11 kN/m

Allowable span between points of support assuming simply supported spans

$$= \left[\frac{8 \times 1.734\ \text{kNm}}{1.11}\right]^{0.5}$$

$$= 3.54\text{m}$$

say = 3.50m

In terms of bending capacity the handrail (without bar) can span up to 3.50m simply supported between points of support. However the allowable span is reduced to **3.0m** in order to keep service load deflection to within 25mm.

Service load deflection of handrail (without bar) for a simply supported span of 3.0m.

$$= \frac{5\ (740 \times 3.0)\ (3000)^3}{384 \times 70000 \times 47 \times (10)^4}$$

$$= 23.72\text{mm}$$

$$= \quad < \quad 25\text{mm} \quad \text{OK}$$

In order that service load deflection shall not exceed 25mm, the simply supported span of the handrail (no bar) is limited to **3.0m**.

Longer balconies with posts:

On longer balconies the handrail (without bar) is used in conjunction with vertical posts installed at **2.70m** maximum spacing to support the handrail.

The posts comprise 60mm x 60mm x 5mm thick square hollow section (SHS).

To allow for deflection of the posts, deflection of the handrail has to be limited so that the overall combined displacement of the handrail + post at any point of the barrier from its original unloaded position does not exceed 25mm. To comply with deflection limitations a maximum post spacing of **2.7m** is adopted.

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Vertical posts

On longer balconies posts are installed at a maximum of **2.70m centres** to support the handrail.

Combined deflection of the posts and handrail has to be limited so that the overall combined displacement at any point of the barrier from its original unloaded position does not exceed 25mm.

For a post spacing of 2.70m service load deflection of the handrail (no bar)

$$\Delta = \frac{5\,F\,L^4}{384\,E\,I} = \frac{5\,(740 \times (2.70)\,(2700)^3}{384 \times 70\,000 \times 47 \times (10)^4} = 15.56\text{mm}$$

60 x 60 x 5mm thick SHS posts: properties of section:

Steel grade	=	S 275 to EN 10025		
Nominal value of yield strength	=	f_y	=	275 N/mm^2
Nominal value of ultimate tensile strength	=	f_u	=	430 N/mm^2
Inertia of section	=	I_{xx}	=	50.50 cm^4
Elastic modulus of section	=	W_{el}	=	16.89 cm^3
Plastic modulus of section	=	W_{pl}	=	20.90 cm^3
Partial factor for material properties	=	γ_{M1}	=	1.10
Partial factor for class 1 sections	=	γ_{MO}	=	1.00
Modulus of elasticity	=	E	=	210 000 N/mm^2

Design ultimate resistance for bending

$$M_{pl,Rd} = \frac{f_y \times W_{pl}}{\gamma_{Mo}} = \frac{275\text{ N/mm}^2 \times 20.90\text{ cm}^3 \times (10)^{-3}}{1.0} = \mathbf{5.75\ kNm}$$

Ultimate moment on post 1.135m high above top of base with posts at 2.70m centres

$$M_d = (0.74 \times 2.70) \times 1.135 \times 1.5\ (\gamma_{Q1}) = 3.40\text{ kNm} < 5.75\text{ kNm} \quad \text{OK}$$

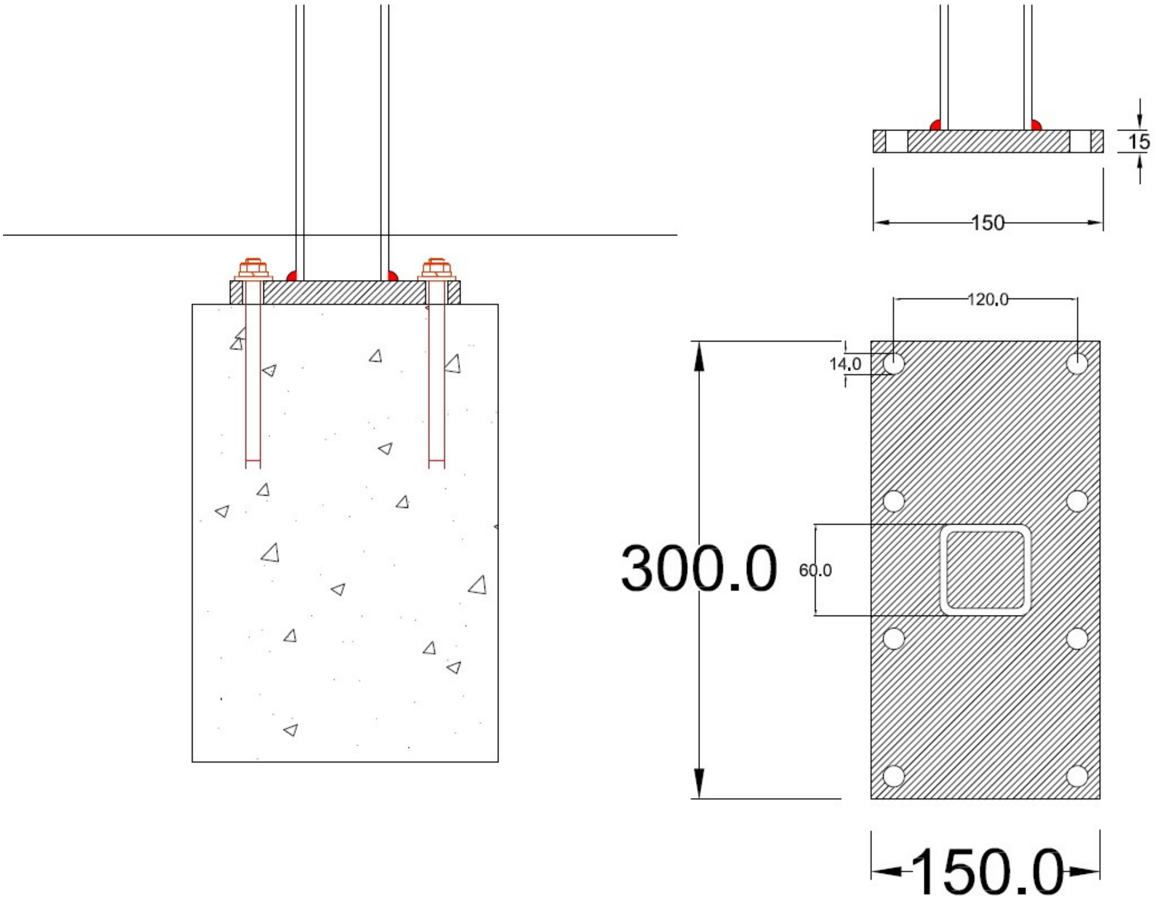
Service load deflection of post supporting 2.70m of handrail

$$\Delta = \frac{P\,L^3}{3\,E\,I} = \frac{(740 \times 2.70)\,(1135)^3}{3 \times 210\,000 \times 50.5 \times (10)^4} = 9.18\text{mm}$$

Combined total displacement of handrail + post from the original unloaded position (service loads)

$$\Delta t = 15.56\text{mm} + 9.18 = 24.74\text{mm} < 25\text{mm} \quad \text{OK}$$

The Balcony 1 system handrail without internal steel reinforcing bar, in conjunction with 60 x 60 x 5mm SHS posts, is adequate to support the design loading on the handrail for posts spaced at up to 2.70 metre centres.



Maximum spacing of posts	=	2.70 m
Ultimate design moment to underside	=	(0.74 kN/m x 1.5) x 2.70 x 1.15
of base on posts at 2.7 m c/c	=	**3.45 kNm**

Base plates and fixing bolts:

Fixing bolts: 8 No. M12:

Lever arm between bolt centres	=	120mm
Ultimate load pull-out force on 4 No. bolts	=	$\frac{(0.74 \times 1.5)\ \text{kN/m} \times 2.70 \times 1.15}{0.12}$
	=	28.72 kN
	=	7.18 kN/bolt (ultimate load)
	=	4.79 kN/bolt (working load)

BS 6180:2011, section 6.5, recommends that barrier fixings, attachments and anchorages should be designed to withstand a greater load than the design loading for the barrier generally. This is intended to ensure that under an extreme load condition, barriers show indications of distress_by distortion, before there is any possibility of sudden collapse due to failure of the fixings. A 50% increase in the design load on fixings is recommended.

Applying the above recommendation, the design **working load** bolt force becomes **7.18 kN/bolt**. This should be within the working load capacity of M12 drilled resin anchor bolts or similar installed into good quality concrete.

Fixing bolts (cont)

Nominal tension capacity of M12 (8.8 grade) bolts			=	37.80 kN/bolt	

Higher bolt forces can therefore be achieved by direct bolting to a substantial steel frame, or by drilling through and anchoring to the underside of a suitable concrete slab.

Base plates: 300 wide x 150 deep x 15mm thick

Bending stresses

Ultimate applied moment on posts at 2.70m maximum spacing	=	M_a	=	(0.74 x 1.5) x 2.70 x 1.135		
			=	3.40 kNm		
Plastic modulus of base	=	W_{pl}	=	$\frac{300 \times (15)^2}{4}$		
			=	16875 mm^3		
Ultimate moment capacity of base	=	Mu	=	$\frac{f_y \times W_{pl}}{\gamma_{M1}}$		
			=	$\frac{275\ N/mm^2 \times 16875mm^3 \times (10)^{-6}}{1.1}$		
			=	4.218 kNm		
Distance from centre of bolts to face of post	=	d	=	30mm		
Ultimate moment on base to face of post	=	M	=	28.72 kN x 0.03		
			=	0.862 kNm		
			= <	4.218 kNm	=	OK

Connection between post & base plate:

The SHS post is welded to the top of the base by means of a full strength butt weld or a continuous fillet weld.

Maximum ultimate elastic bending stress on post	=	$\frac{M}{W_{el}}$	=	$\frac{3.40 \times (10)^6}{16.89 \times (10)^3}$	=	201.3 N/mm^2
					=	1.006 kN/mm on 5mm section
Transverse capacity of 6mm fillet weld			=	1.155 kN/mm).	=	OK

Posts to be connected to base plates using full strength butt welds, continuous 6mm fillet welds, or a combination of welds that achieves a full strength connection.

Glass infill:

Design standard = Institution of Structural Engineers publication *'Structural use of glass in buildings (second edition) February 2014'.*

Glass type = 10mm thick thermally toughened soda silicate safety glass with smooth float 'as produced' finish with polished edges.

Characteristic design strength = 120 N/mm^2

$$f_{g;d} = \frac{K_{mod} \times K_{sp} \times K_{g;k}}{\gamma_{M;A}} + \frac{K_v\,(f_{b;k} - f_{g;k})}{\gamma_{M;V}}$$

where:

K_{mod} = 30 second load duration factor
= 0.89 for a domestic balustrade load

K_{sp} = glass surface profile factor
= 1.0 for float glass 'as produced'

$f_{g;k}$ = characteristic strength of basic annealed glass
= 45 N/mm^2

K_v = manufacturing process strengthening factor
= 1.0 for horizontal toughening

$f_{b;k}$ = characteristic bending strength of prestressed glass (120 N/mm^2)

$\gamma_{M;A}$ = material partial factor
= 1.6 for basic annealed glass

$\gamma_{M;V}$ = material partial factor
= 1.2 for surface prestressed (toughened) glass

Ultimate design stress $f_{g;d}$

$$= \frac{0.89 \times 1.0 \times 45}{1.6} + \frac{1.0\,(120 - 45)}{1.2}$$

= **87.53 N/mm^2**

Section modulus of glass 10mm thick Z

$$= \frac{1000 \times (10)^2}{6} = 16667 \text{ mm}^3/\text{m}$$

Ultimate moment capacity of glass 1000mm wide x 10mm thick Mu

= $f_{g;d} \times Z$

= 87.53 N/mm^2 x 16667mm^3 x (10)$^{-6}$

= **1.459 kNm/m**

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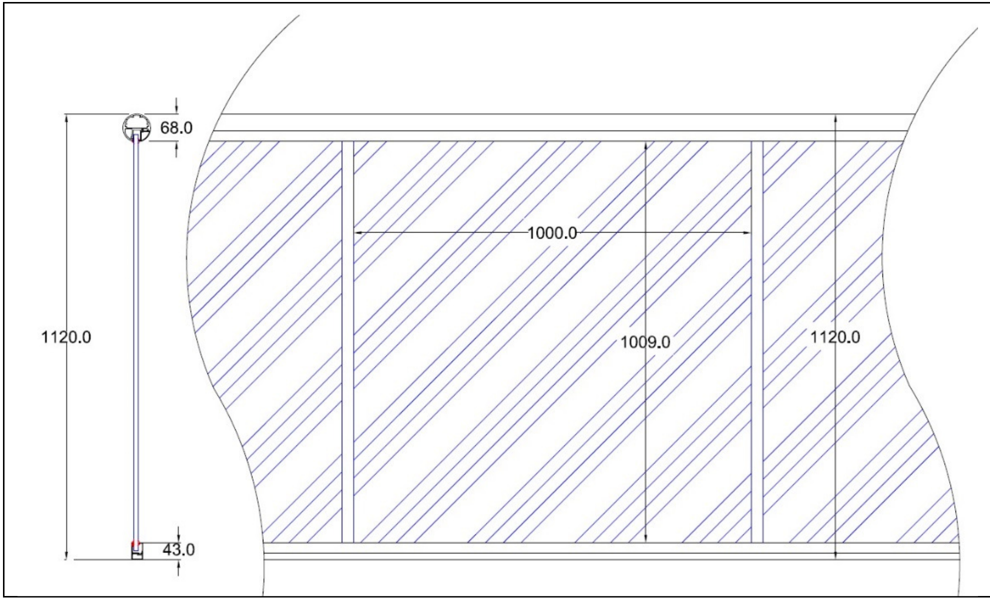


Glass panels can be any length.

For the purposes of design and checking a nominal glass panel width of 1000mm simply supported between the bottom rail and the handrail has been used.

Separate design loading conditions are considered:

1. Uniformly distributed service wind load on the infill of 1.35 kN/m^2

Ultimate UDL on glass w = 1.35 kN/m^2 x 1.5 = 2.025 kN/m^2

Ultimate moment on glass due to UDL on span of 1.0m Mu = $\frac{2.025 \text{ kN/m}^2 \times (1.0)^2}{8}$ = 0.253 kNm/m

= < 1.459 kNm = OK

2. Point service load on the infill of 0.5 kN

Ultimate point load on the glass = 0.75 kN point load applied in any position

Worst case for bending stress on the glass due to point load = point load applied at mid-height of glass

Ultimate moment on glass due to point load = $\frac{0.75 \text{ kN} \times 1.5 \times 1.0\text{m}}{4}$ = 0.281kNm

Conservatively, it is assumed that this bending moment is carried by a 300mm wide vertical strip of glass.

Moment capacity of 300mm strip = 1.459 kNm x 0.3 = 0.4377 kNm

= > 0.1875kNm = OK

The glass is adequate to support the ultimate design loading in terms of bending capacity.

Glass deflection:

1. Overall UDL:

Service load deflection due to the design overall UDL:

Inertia of glass 10mm thick x 1000mm long = $\frac{1000 \times (10)^3}{12}$ = 83333 mm^4

Service load deflection due to a UDL of 1.35 kN/m^2 on a simply supported span of 1.0m = $\frac{5\,w\,L^4}{384\,E\,I}$

= $\frac{5 \times (1350 \times 1.0)\,(1000)^3}{384 \times 70\,000 \times 83333}$ = 3.01mm = OK

2. Point load:

Conservatively, for deflection calculation purposes consider that the design point load is carried by a 300mm wide vertical strip of glass:

Inertia of glass 10mm thick x 300mm long = 0.3 x 83333 mm^4 = 25 000 mm^4

Service load deflection due to a point load of 0.5 kN applied at mid-span = $\frac{P\,L^3}{48\,E\,I}$

= $\frac{500 \times (1000)^3}{48 \times 70\,000 \times 25\,000}$

= 5.95mm < $\frac{span}{65}$ = OK

The glass is adequate in terms of both bending strength and deflection.

Wall fixings:

The handrail wall fixing consists of 3mm thick stainless steel angles bolted to the wall with 2 No. M8 stainless steel resin anchors and secured to the handrail with 2 No. 4.8mm diameter stainless steel Phillips self-tapping screws.

The allowable simply supported span of the **handrail (with bar)** between points of support is **3.3m**.

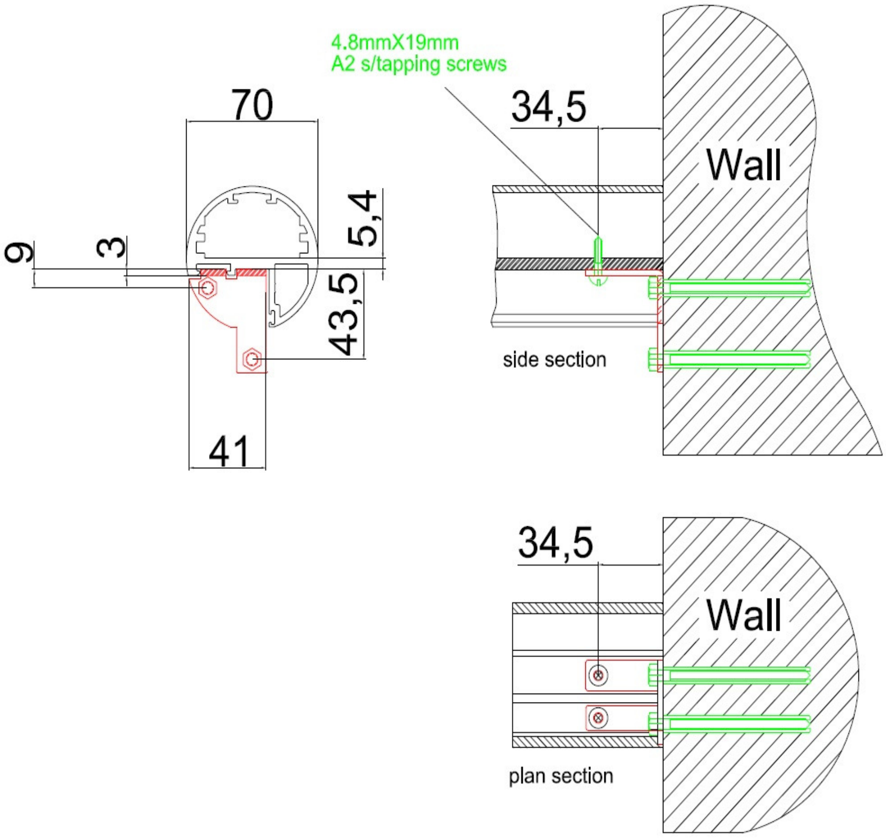
Horizontal service (working) load on the wall fixing for a span of 3.3m = 0.74 kN/m x 1.65m = 1.221 kN/fixing

The horizontal load on the handrail is applied to the fixing angles at the position of the Phillips screws located 34.5mm from the back of the angles. The wall fixing bolts are 24mm apart horizontally.

Pull-out forces on wall fixing

Working load pull-out force on the anchor bolts = 1.221 kN x $\frac{34.5}{24}$ = 1.755 kN/bolt

Applying the 50% increase on fixing design loads recommended in BS 6180:2011, this becomes **2.63 kN/bolt**.



Wall fixing detail

The allowable simply supported span of the handrail (with bar) between points of support is 3.3m.

Horizontal service (working)	=	0.74 kN/m x 1.65m
load on the wall fixing for a	=	1.221 kN/fixing
span of 3.3m		

__**Shear forces on wall fixings: handrail (with bar):**__

Working load shear force on the anchor bolts and the 4.8mm x 19mm stainless steel self-tapping screws	=	1.221 kN/2	=	0.61 kN/bolt or screw

Applying the 50% increase in design load on anchor bolts recommended in BS 6180:2011, this becomes **0.915 kN/bolt**.

For the **handrail (without bar)** the allowable simply supported span is **3.0m.** Design forces on the anchor bolts are therefore reduced by 3.0/3.3. ie. working load pull-out force = 2.63 x 3.0/3.3 = **2.39 kN/bolt.** Working load shear force on the anchor bolts = 0.915 x 3.0/3.3 = **0.83 kN/bolt.**

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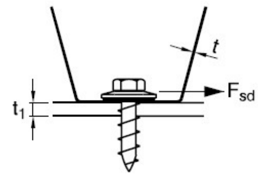
<u>Phillips stainless steel self-tapping screws</u>

Shearing force, construction screws

Dimensioning value F_{sd} kN/screw. Attention is paid both to failure of the edge of the hole and shearing failure in the screw. Safety class 1.



Nom t mm	When calculating t mm	Tensile yield limit N/mm²	Screw diameter 4.2 mm		Screw diameter 4.8 mm		Screw diameter 5.5 mm		Screw diameter 6.3 mm	
			t_1 =t	t_1 = 2.5 t	t1 =t	t_1 = 2.5 t	t_1 =t	t_1 = 2.5 t	t_1 =t	t_1 = 2.5 t
0.4	0.32	250	0.26	0.54	0.28	0.61	0.30	0.70	0.32	0.81
0.5	0.41	250	0.38	0.69	0.40	0.79	0.43	0.90	0.46	1.03
0.6	0.52	250	0.52	0.86	0.56	0.98	0.60	1.12	0.64	1.29
0.7	0.60	350	0.93	1.41	1.00	1.61	1.07	1.85	1.14	2.12
0.8	0.73	350	1.25	1.72	1.34	1.96	1.43	2.25	1.53	2,58
1.0	0.93	250	1.29	1.56	1.38	1.79	1.47	2.05	1.58	2,34
1,0	0,93	350	1,80	2,19	1,93	2,50	2,06	2,86	2,21	3,28
1,2	1,13	350	2,41	2,66	2,58	3,04	2,76	3,48	2,95	3,99
1,5	1,42	250	2,39	2,39	2,60	2,73	2,78	3,12	2,97	3,58
1,5	1,42	350	3,03*	3,03*	3,63	3,82 3,64	3,89	4,37	4,16	5,01
2.0	1.91	350	3.03*	3.03*	4.16 3.64	4.16 3.64	5.72 5.20	5.72 5.20	6.49	6.74
2.5	2.40	350	3.03*	3.03*	4.16 3.64	4.16 3.64	5.72 5.20	5.72 5.20	7.80 6.76	7.80 6.76

In the area of number pairs in the table and marked *, shearing failure in the screw is decisive.
The value to the left in each number pair relates to carbon steel screws, while the number to the right relates to stainless steel screws.

Excerpt of the table at the foot of page 7 of Lindab's literature headed 'Shearing force, construction screws'

<u>Properties of stainless steel for angle brackets and self-tapping screws:</u>

Material type = stainless steel grade 304
Characteristic ultimate tensile strength = 621 N/mm²
Characteristic 0.2% proof stress = 290 N/mm²

Phillips self-tapping screws: ultimate shear loads taken from the table in Lindab's technical literature.
Thickness of aluminium in the handrail at screw positions = 5.4mm

Thickness of stainless steel angle brackets (Nom t mm) = 3.0mm

Ultimate shear capacity of 4.8mm diameter screws, safety class 1 for Nom t = 2.5mm = 3.64 kN/screw (from Lindab's table)

For safety classes 2 and 3 this value is divided by 1.1 and 1.2 respectively. Safety class 3 is the highest safety class and has been assumed to apply to balustrades. The shear capacities given in Lindab's table are based upon material having a tensile yield limit of 350 N/mm^2. The values given in the table have been adjusted to allow for the yield stress of stainless steel type 304 (290 N/mm^2.)

The ultimate shear capacity of 3.64 kN/screw has therefore been reduced by 290/350 and divided by 1.2 to represent safety class 3 and 290 N/mm^2 yield stress rather than 350 N/mm^2. The adjusted ultimate shear capacity is then **2.51 kN/screw.**

Ultimate shear force/screw	=	1.37 kN < 2.51kN	=	OK

Stainless steel brackets

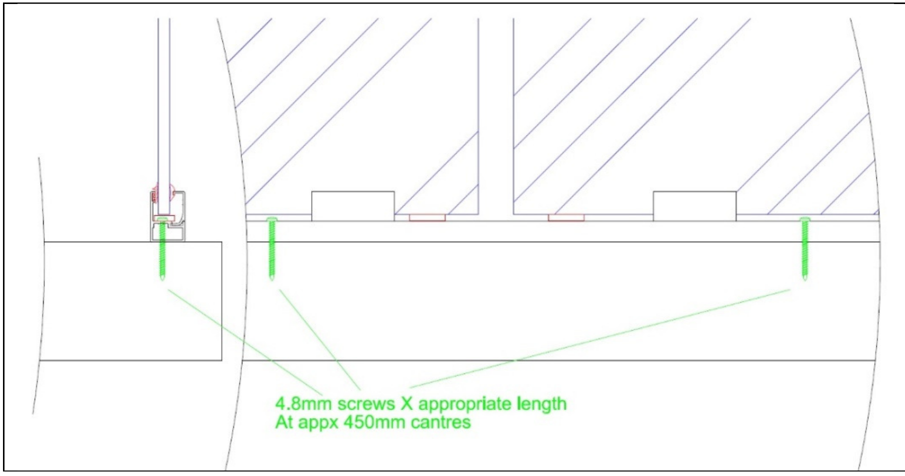
The top part of the bracket consists of two separate sections: one 15.5mm x 3mm and one 13mm x 3mm.

Plastic modulus of 15.5 x 3mm section for lateral loads	=	$\frac{3 \times (15.5)^2}{4}$	=	180.19 mm^3	
Plastic modulus of 13 x 3mm section for lateral loads	=	$\frac{3 \times (13)^2}{4}$	=	126.75 mm^3	
Σ plastic modulus	=	306.94 mm^3			
Σ resistance moment	=	290 N/mm^2 x 306.94 mm^3 x $(10)^{-6}$			
	=	0.089 kNm			
For a simply supported span of 3.3m: Ultimate load on end bracket	=	1.11 kN/m x 1.65	=	1.83 kN	

This load is applied 34.5mm from the face of the bracket.

Moment applied horizontally to the top of the bracket	=	1.83 x 0.0345	=	0.063 kNm	
			= <	0.089 kNm	OK

Bottom rail fixings:



The standard bottom rail fixing consists of 4.8mm diameter screws at 450mm centres.

The worst case for design loading on the fixings is when the design service point load of 0.50 kN on the glass acts at the position of a fixing. The fixing screw is then subjected to a working load shear force of 0.50 kN/screw.

Balcony 1 system (0.74 kN) ***PAGE 17*** **(B1NB6060300150BP141117R)**

Bottom rail fixings:

The allowable load on the fixing screws varies depending upon the type and thickness of the material into which the screws are inserted.

As an example, fixing to a balcony deck comprising 15mm thick plywood strength class C16, group 1, the basic allowable working load single shear value given in BS 5268 : Part 2 : 1996 for a No. 10 (4.88mm) screw 45mm long is 0.519 kN.

Where a pre-drilled steel component of adequate strength is screwed to a timber member, the basic lateral load of 0.519 kN is multiplied by a modification factor of 1.25, making an allowable shear value of 0.648 kN, which is adequate in relation to the design working load shear force of 0.50 kN.

Other values of allowable shear loads on fixings will apply where the deck material is of different strength and/or thickness.

The installers should satisfy themselves that the fixings chosen are adequate to resist the design loads in relation to the fixing material in each individual installation.

SUMMARY
Orbit (Balcony 1) system (with and without 58 x 4mm steel internal reinforcing bar) using 60 x 60 x 5mm SHS posts and 300 x 150 x 15mm baseplates

1) On single span and corner balconies, the **handrail (without internal reinforcing bar)** is capable of supporting the design ultimate imposed and wind loads over spans up to **3.0 metres** between points of support. (i.e. a handrail wall fixing, or a handrail corner joint.) The **handrail (with internal reinforcing bar)** is capable of supporting the design factored loads on single span and corner balconies on spans up to **3.3 metres** between points of support.

2) On longer balconies where the length of the balustrade exceeds 3.0 metres, the handrail without internal reinforcing bar is used in conjunction with vertical posts installed at a maximum spacing of **2.70m** between post centres. The posts comprise **60 x 60 x 5mm** square hollow steel sections (SHS) in steel grade S 275 H. The posts are welded to **300 x 150 x 15mm** base plates by means of full strength butt welds, continuous 6mm fillet welds **(6 FW),** or a combination of welds that achieves a full strength connection**.** 14mm diameter holes are provided for **8 M12** holding down bolts.

3) For the maximum span of **3.0 metres** on single span and corner balconies using the handrail without internal reinforcing bar, the design horizontal working pull-out load on the wall fixing bolts is **2.39 kN/bolt**. The horizontal working shear load on the wall fixing bolts is **0.83 kN/bolt**.

4) For the maximum span of **3.3 metres** on single span and corner balconies using the handrail with internal reinforcing bar, the design horizontal working load pull-out load on the wall fixing bolts is **2.63 kN/bolt**. The horizontal working load shear force on the wall fixing bolts is **0.915 kN/bolt.** These loads should be achievable using drilled resin anchor bolts or similar into good quality concrete or brickwork.

5) On longer balconies, where the handrail (without bar) is used in conjunction with posts installed at a maximum spacing of **2.70m**, the design **working load** pull-out force on the holding down bolts is **7.18 kN/bolt.** This load should be achievable using drilled resin anchor bolts or similar.

6) The installers should satisfy themselves that the fixing bolts chosen are suitable to resist the specified loads, and also that the structure into which they are installed can support these loads.

7) The 4.8mm diameter self-tapping stainless steel screws connecting the handrail to the stainless steel angle brackets at wall and post fixings are adequate to support the design loads specified in relevant British and European Standards. The 3mm thick stainless steel brackets are also adequate to support these loads.

8) The standard bottom rail fixing comprises 4.8mm diameter screws inserted into the balcony deck at 450mm centres. At this spacing the fixings are required to have a working load shear capacity of 0.50 kN/screw. The installers should satisfy themselves that the screws chosen are suitable to resist this load when inserted into the particular deck material present on a specific project. Where the deck material is of reduced strength and/or thickness the spacing of the screws should be reduced accordingly.

9) The 10mm thick thermally toughened safety glass infill panels are adequate to support the design loads specified in the relevant British and European Standards.

Prepared for and on behalf of Balconette by
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